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units convenient for LTS design. The algebraic form of

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these equations is somewhat different ; however, the behavior is similar as illustrated in Figure C3.8.7-1 and C3.8.7-1 and C3.8.7-2 where different shapes and equations are shown. The typical extreme event wind speed is 105 mph or greater.

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$z =$ AASHTO LTS Eq. 3.8.4-1 Height and Exposure

Factor for Pole $K_z =$ AASHTO LTS Eq. 3.8.4-1

Directionality Factor $K_d =$ AASHTO LTS 3.8.5 Gust

Effect Factor $G =$ AASHTO LTS 3.8.6 Velocity

Conversion Factor - Ext Event $C_{v-Ext} =$ AASHTO LTS

3.8.7 $C_v V_d = C_v V_{\phi}$ pole-avg = Velocity

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Conversion Factor $C_v = \text{AASHTO LTS 3.8.7 } C_v V d = C_v V \emptyset \text{ pole ...}$

~~APPENDIX A EXAMPLE 10 SIGN STRUCTURE FOUNDATION DESIGN~~

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(LTS-6) AASHTO has issued an errata that includes technical revisions to the Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, Sixth Edition. To ensure that your editions are accurate and current, we are providing you with the attached summary of the

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AASHTO LTS- 6 strength and fatigue criteria, using

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these criteria and methods to study and evaluate the Maryland Department of Transportation's State Highway Administration sign structure database and taking samples from the selected categories. The research developed a methodology for analyzing sampled sign

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